

# Rigid Pavement Climatic Effects in Illinois

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# Acknowledgements

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- ICT R57 – Technical Review Panel (2005-08)
  - Illinois Department of Transportation
    - Amy Schutzbach et al.
  
- UIUC Students
  - Jake Hiller
  - Dong Wang
  - Victor Cervantes
  - Matt Beyer
  - Amanda Bordelon

# Overview

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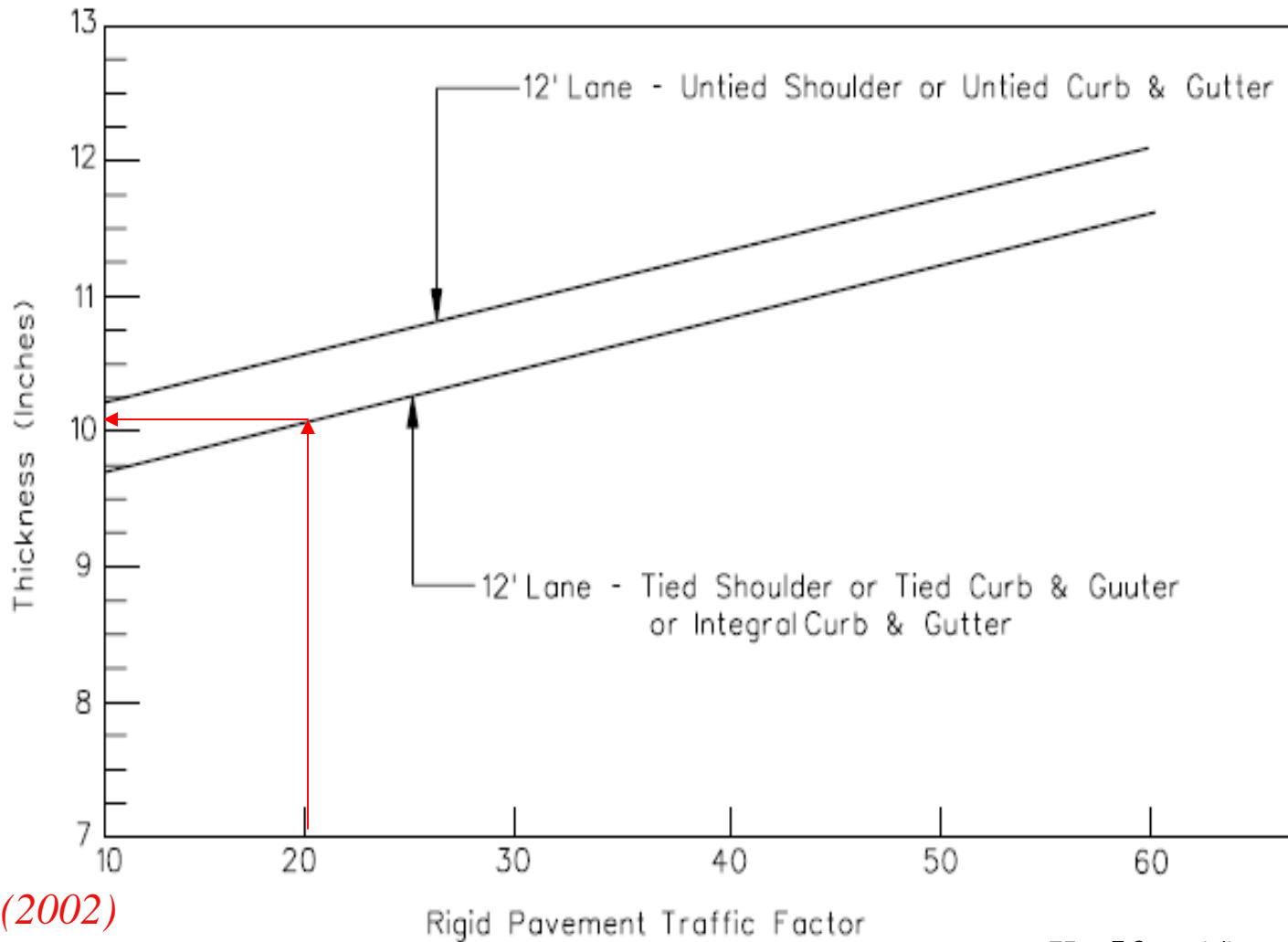
- Illinois has existing M-E JPCP method by Zollinger and Barenberg (1989)
  - No direct climate consideration
  
- IDOT has an semi-empirical method to determine CRCP thickness
  - No direct climate consideration
  
- Update/refine existing JPCP procedure and develop M-E CRCP design method

# Existing IDOT JPCP Method

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- Traffic = ESALs
- MOR = 703 psi (?)
- k-value = 50, 100, 200 psi/in
- Temperature curling (k=100 psi/in)
- Joint Spacing = 15ft
- Shoulder Type = AC or Tied [widen]
- Reliability (95% curves)
- Failure = 20% slabs cracked – TF>10
- COPES data calibration

# IDOT M-E JPCP Method



IDOT (2002)

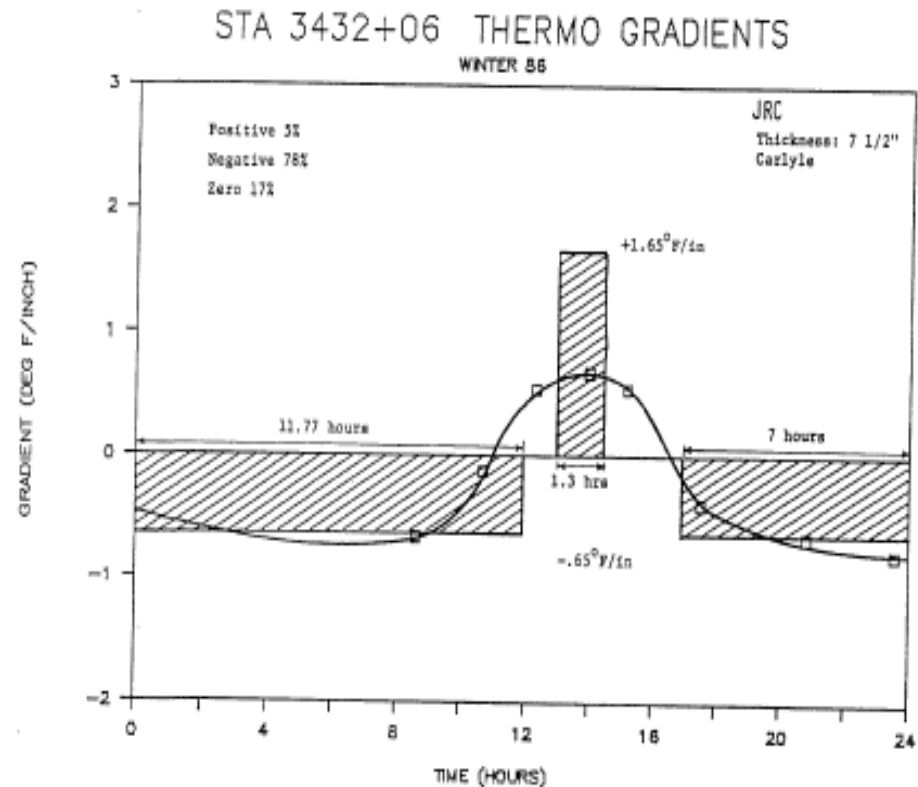
$K=50 \text{ psi/in}$

# IDOT assumed Thermal Gradients

35% Night ( $-0.65^{\circ}\text{F}/\text{in}$ )

25% Day ( $+1.65^{\circ}\text{F}/\text{in}$ )

40% Zero ( $0^{\circ}\text{F}/\text{in}$ )



# M-EPDG Evaluation

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- Objective
  - Evaluate version 0.91 vs. 1.0
  - Determine effect of Climate on PCC thickness in Illinois
  - Is there a need for a geography / climate-based design method in Illinois?

# Concrete Coefficient of Thermal Expansion (COTE)

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- Illinois SHRP Test Sites
  - 84 total cores
  
- $AVERAGE_{80\%} = 5.7 \times 10^{-6}/^{\circ}F$  (69 cores)
- $STD\ DEV_{80\%} = 0.33 \times 10^{-6}/^{\circ}F$
- $COV = 6\%$

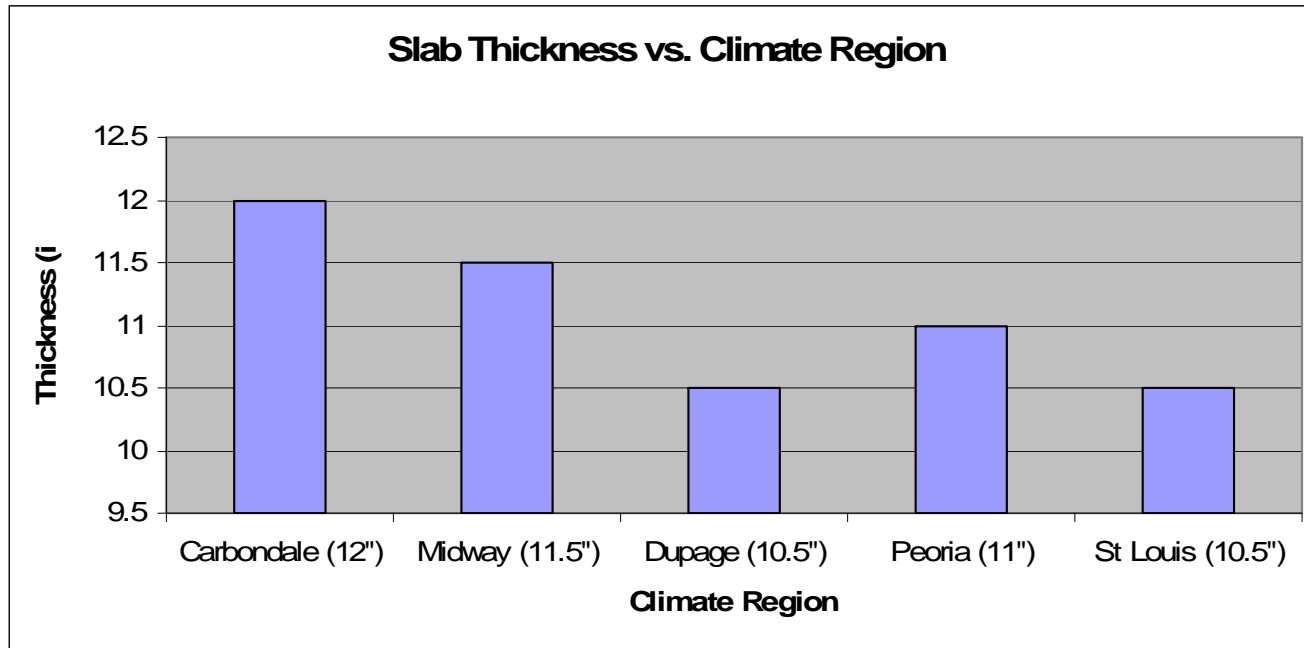


# Climate Effect Inputs

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- Changes in Climatic Effects
  - Climate data for several Illinois cities ran with E-ICM
  
- Concrete thickness was changed to ensure less than 20% slab cracking for each climate
  - *No faulting or IRI criteria limit!*

# Climatic Effects (v. 0.91)



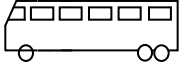

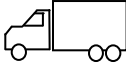

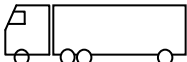
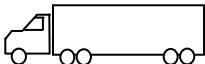
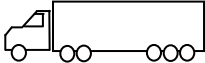
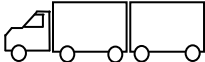
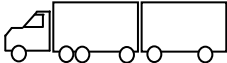

- Five regions in Illinois
- Range of slab thickness – 10.5” to 12”
- Pavement at all sites had less than 20% cracking at 30 yrs

# V.1.0 MEPDG / IDOT Inputs

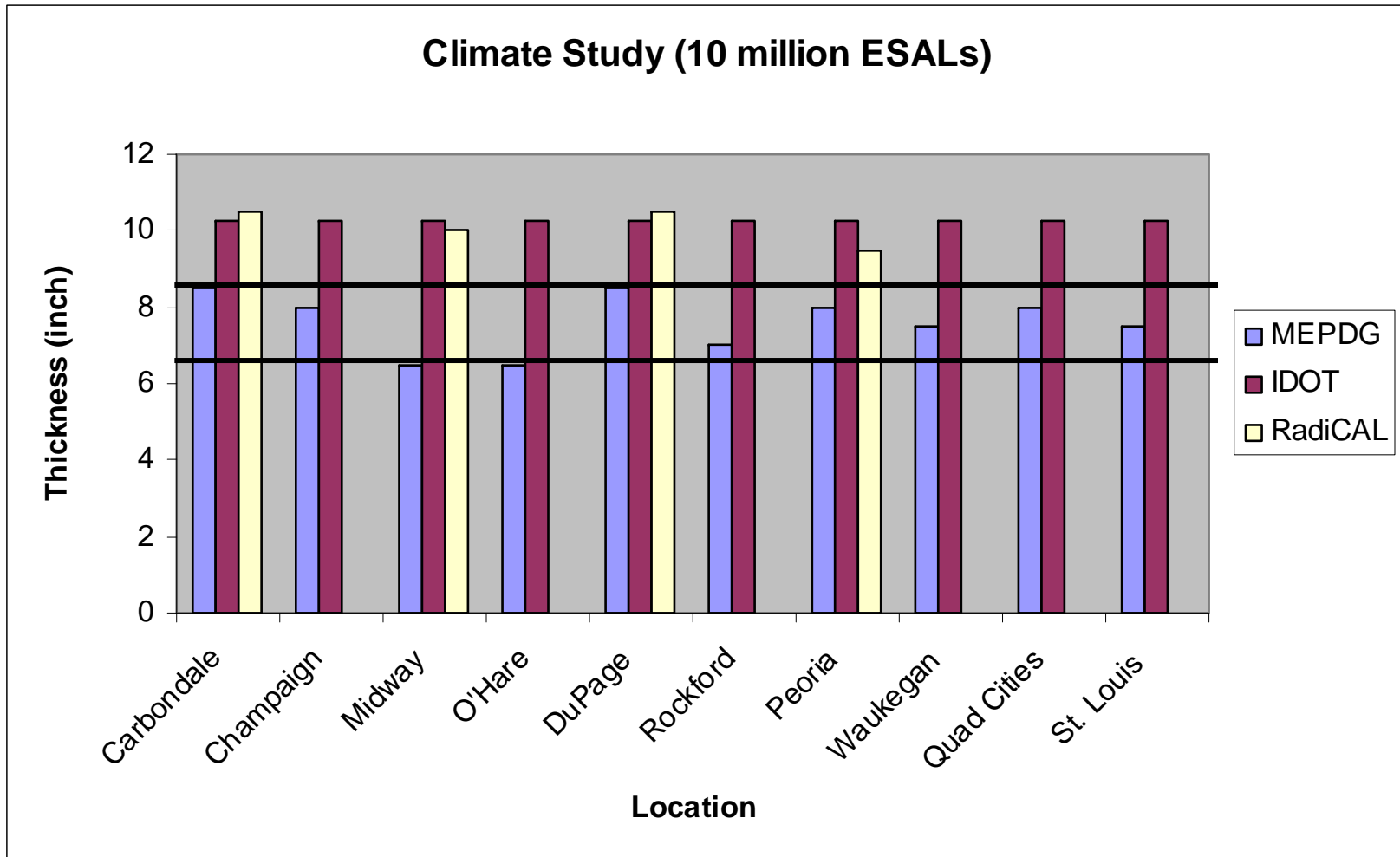
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- MEPDG (v1.0) default load spectra (TTC1)
- Illinois Vehicle Class Distribution
- Variables
  - Shoulder type (AC, tied, widen lane)
  - slab length (12, 15, 18 ft)
  - fatigue algorithm (MEPDG)
  - temperature profile (linear, nonlinear)
  - built-in curl (-10°F)

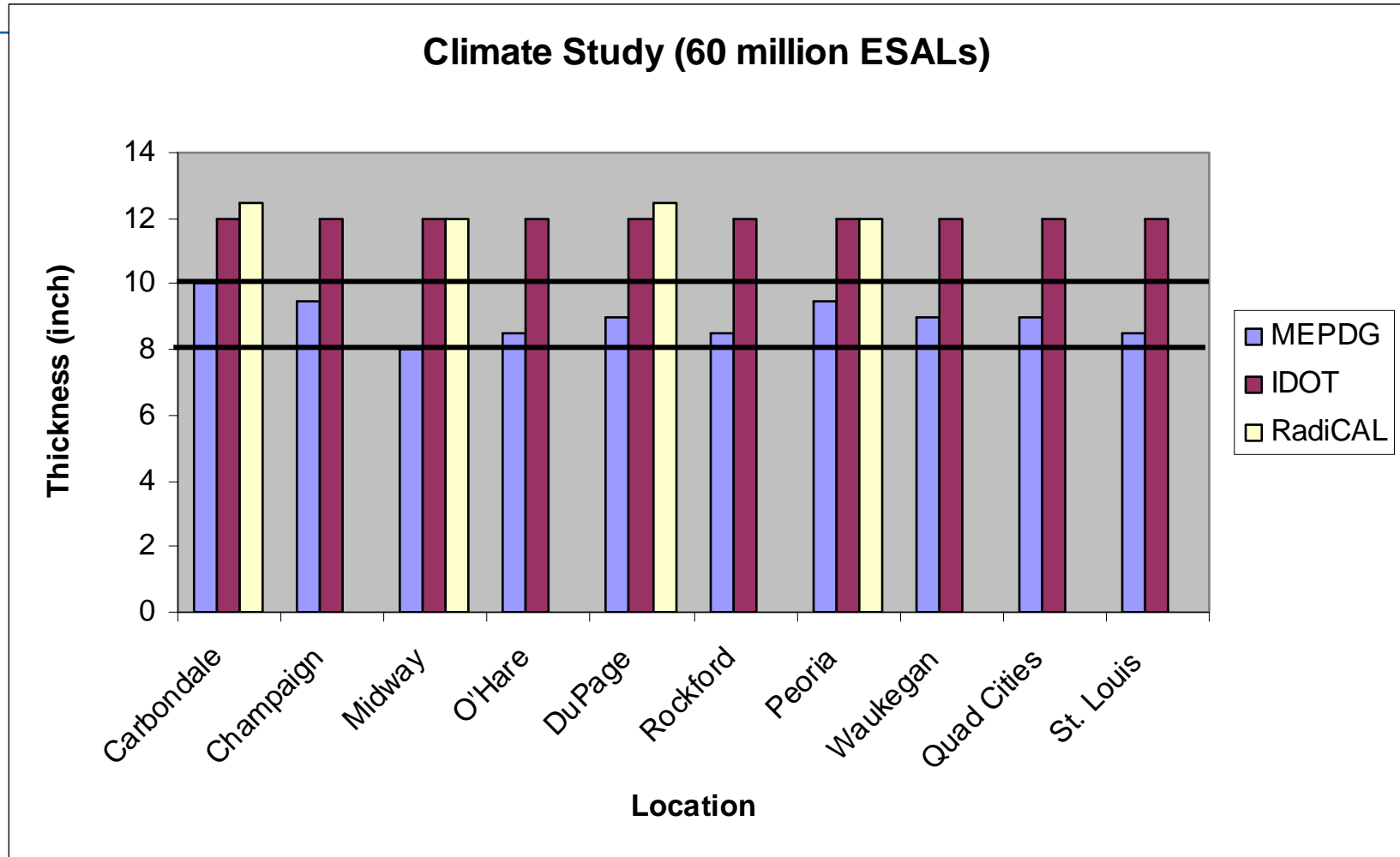
# Vehicle Class distribution

<u>Class</u>	<u>Illinois</u>	<u>California</u>	<u>M-EPDG</u>	
Class 4	1.4%	1.1%	1.8%	
Class 5	3.8%	23.0%	24.6%	
Class 6	2.3%	5.2%	7.6%	
Class 7	0.0%	0.3%	0.5%	
Class 8	3.8%	6.7%	5.0%	
Class 9	84.4%	50.6%	31.3%	
Class 10	0.5%	0.6%	9.8%	
Class 11	2.8%	8.8%	0.8%	
Class 12	0.3%	1.1%	3.3%	
Class 13	0.3%	0.1%	15.3%	

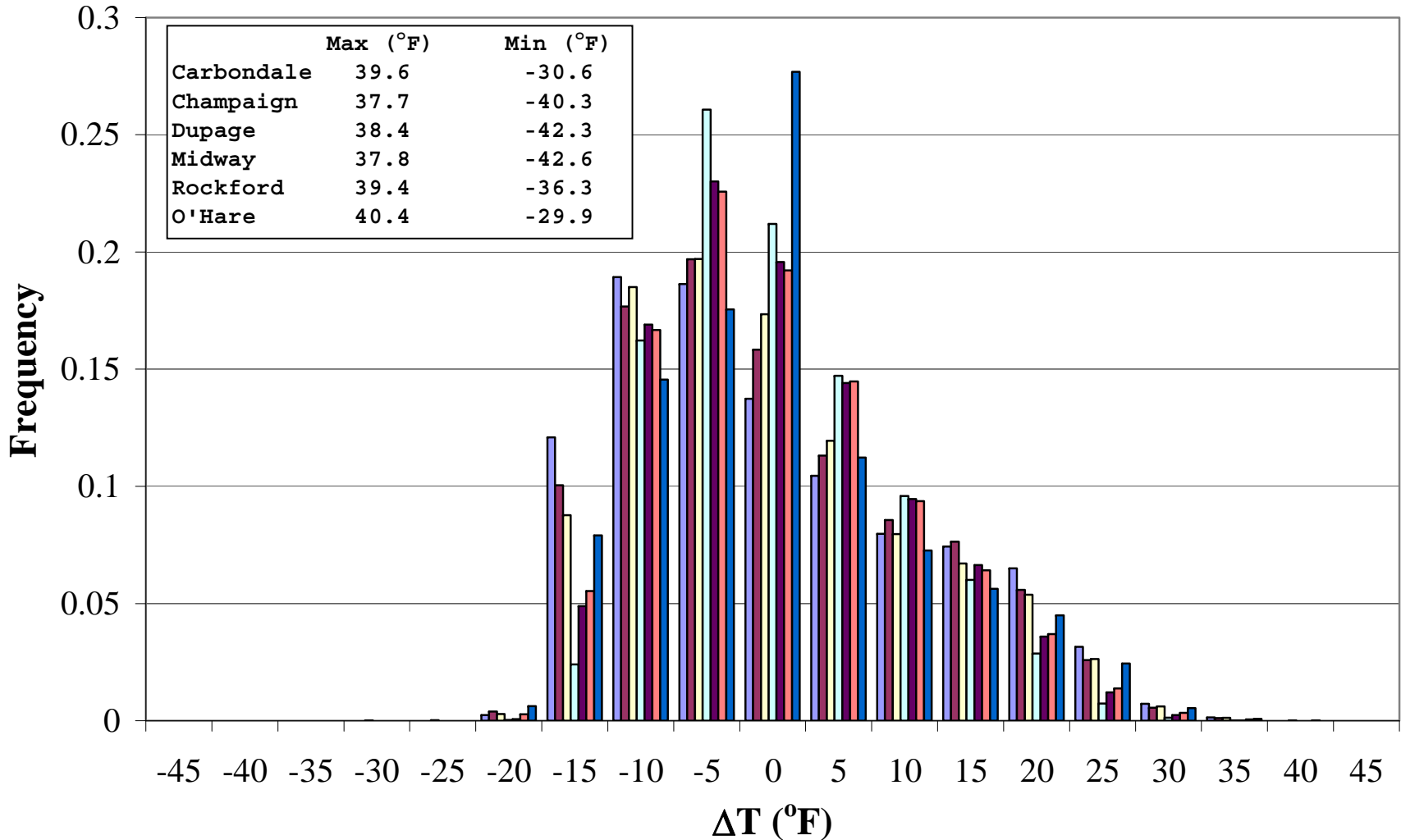
# Climate Study – $10 \times 10^6$ ESALs



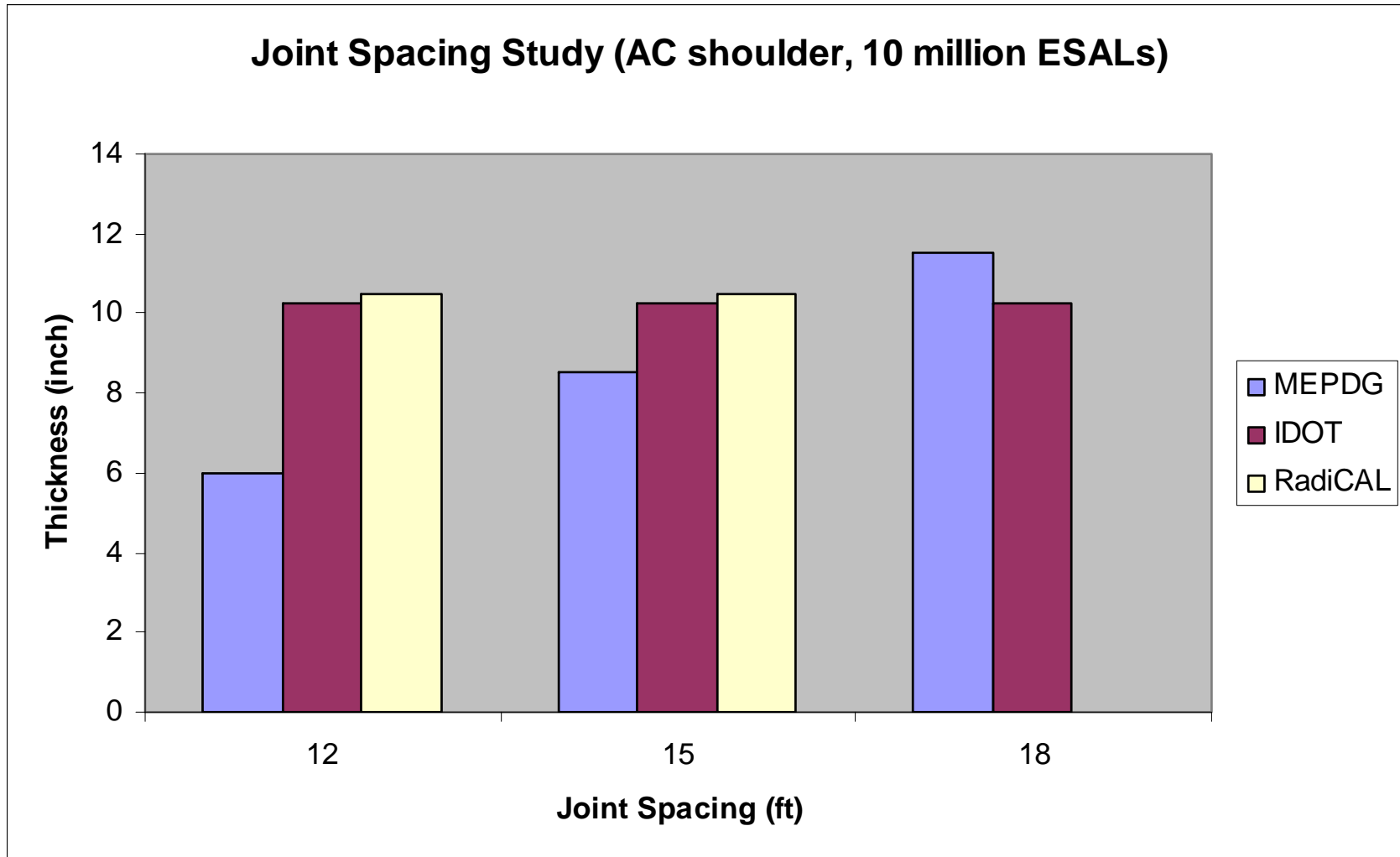
# Climate Study – $60 \times 10^6$ ESALs



# Temperature Differential Freq.

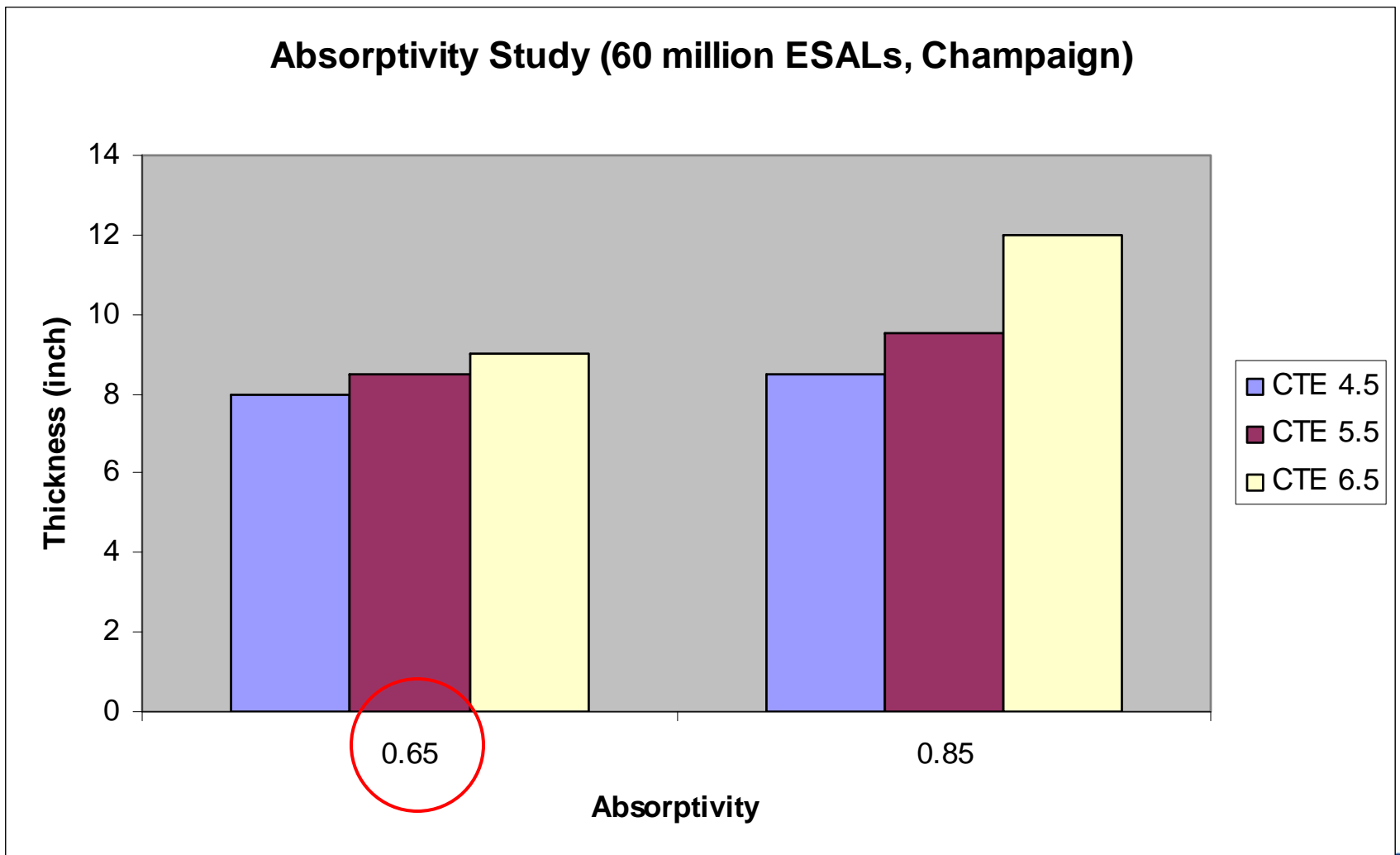


# Joint Spacing – 10M ESALs and AC Shoulder





# Thermal Properties



# Findings – CLIMATE -JPCP

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## Climate

- Sensitive (1.5” to 2”)
  - How to accommodate?
- 
- Temperature Curling
    - Nonlinear is more representative

# (IL) Climatic Zone Consideration

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- Separate CHART for state zones (?)

## Design Feature limitations ( $h > 10$ inches)

- $\leq 15'$  south of I-80?
- 18' use structural fibers or higher specified strength
  
- For  $h \leq 10$  inches
- 12' south of I-80?

# Initial MEPDG (v1.0) CRCP Analysis

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## □ Concrete Materials

- MOR = 585 psi at 28 days (3<sup>rd</sup> point bending)
- Cement content: 550 lbs/cy (w/c=0.42)
- COTE =  $5.5 \times 10^{-6} / ^\circ\text{F}$  (*absorbitivity*=0.85)

## □ Reinforcement

- 20-year: 0.7% steel, #6 bars
- 30-year: 0.8% steel, #7 bars
- steel depth:
  - 3.5" for 10, 60 million ESALs
  - 4.5" for 230 million ESALs

## □ -10°F Built-in Curl

# Traffic Inputs

- Bolingbrook Data
  - vehicle class distribution
  
- M-EPDG Default Values
  - hourly adjustment
  - axle load distribution
  - # of axle types/truck class
  
- Tire pressure = 80 psi

Vehicle Class	Bolingbrook (NB)
4	1.6%
5	4.6%
6	3.7%
7	0.0%
8	6.7%
9	79.0%
10	0.9%
11	3.5%
12	0.0%
13	0.0%

# CRCP Traffic Assumptions

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AADTT values for MEPDG v1.0

## 20-year design

10 million ESALs = 1,657 AADTT

60 million ESALs = 9,918 AADTT

230 million ESALs = 38,021 AADTT

## 30-year design

10 million ESALs = 1,105 AADTT

60 million ESALs = 6,612 AADTT

230 million ESALs = 25,347 AADTT

***AADTT = Average Annual Daily Truck Traffic***

# Design Features

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- *PCC thickness is design variable*
- Asphalt concrete base = 4 inch
- A-7-6 soil ( $E = 7,500$  psi)
  
- Crack spacing = calculate
  
- Construction month = August

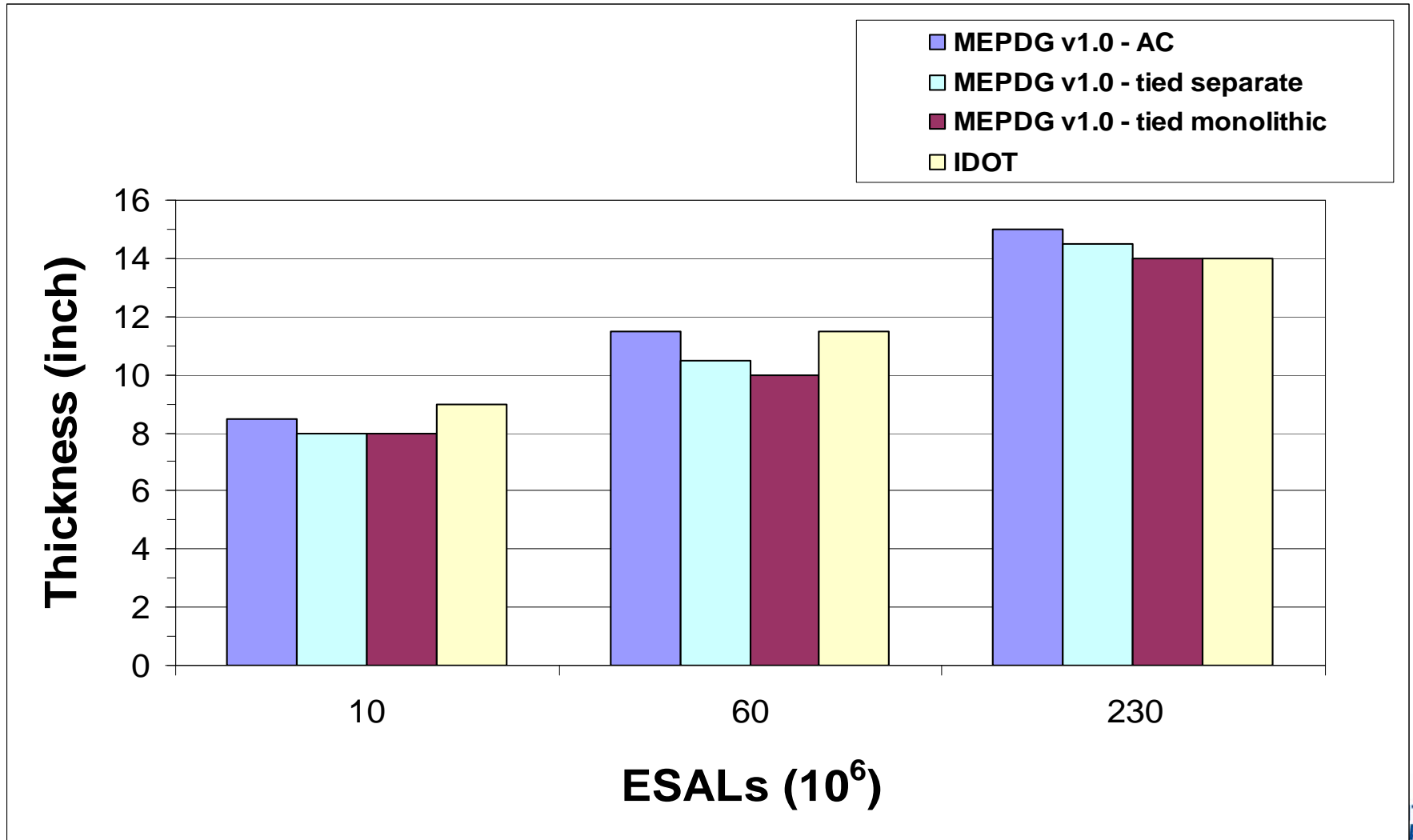
# Failure Criteria

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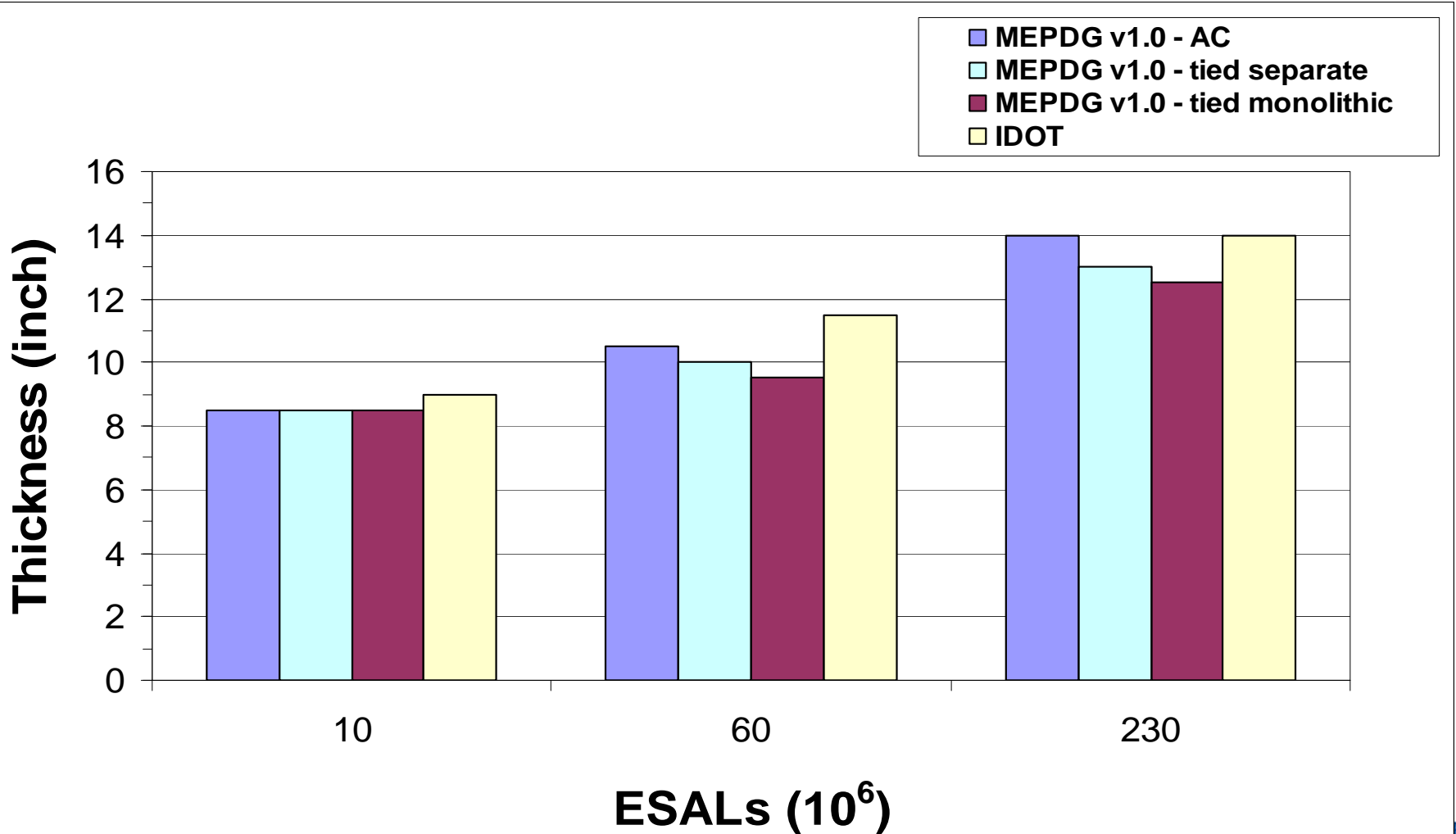
- Punchout = 10/mile @ 95% reliability
- IRI = ignore this failure criteria



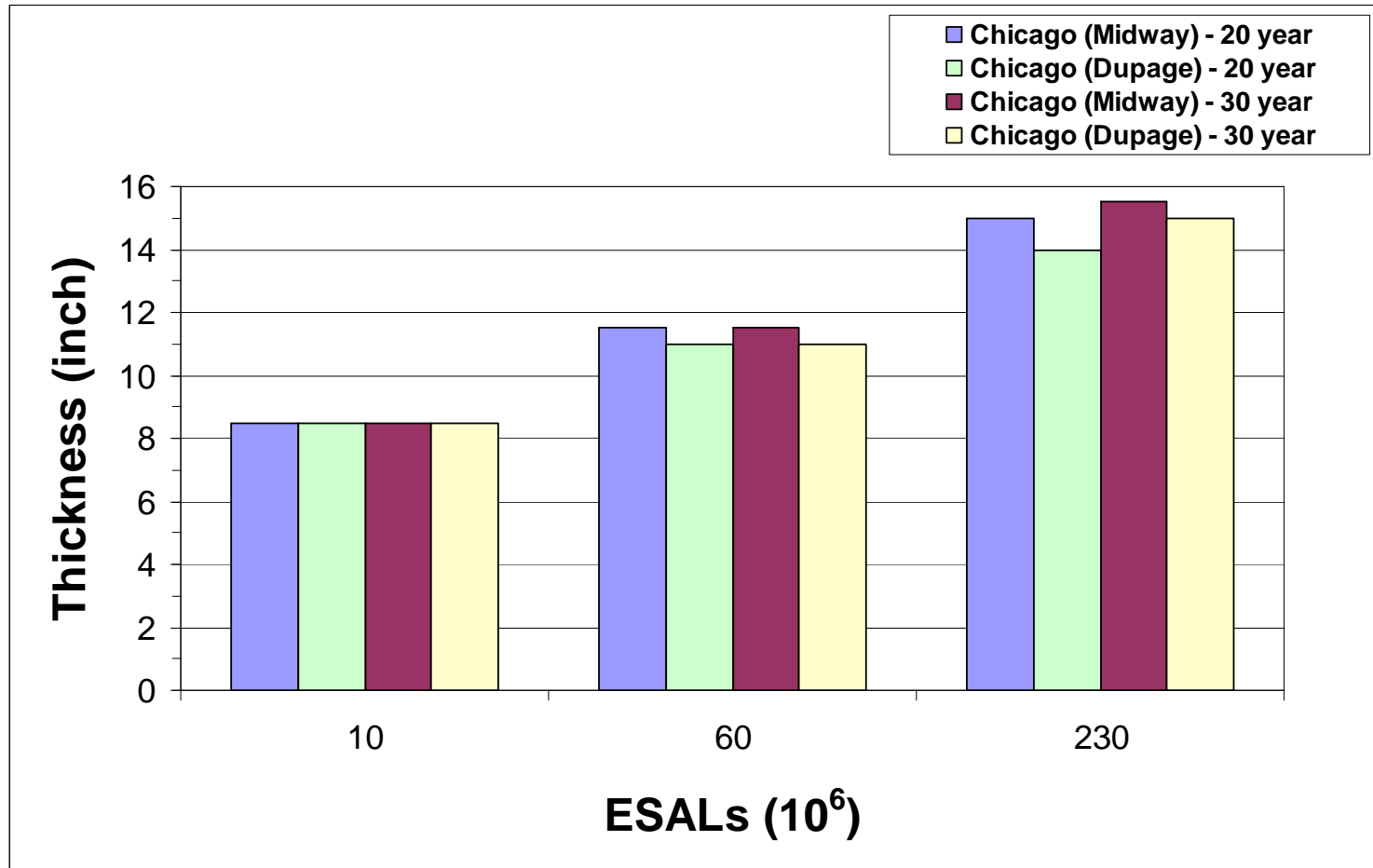
# Chicago (Midway) [20 year]



# Carbondale [20 year]

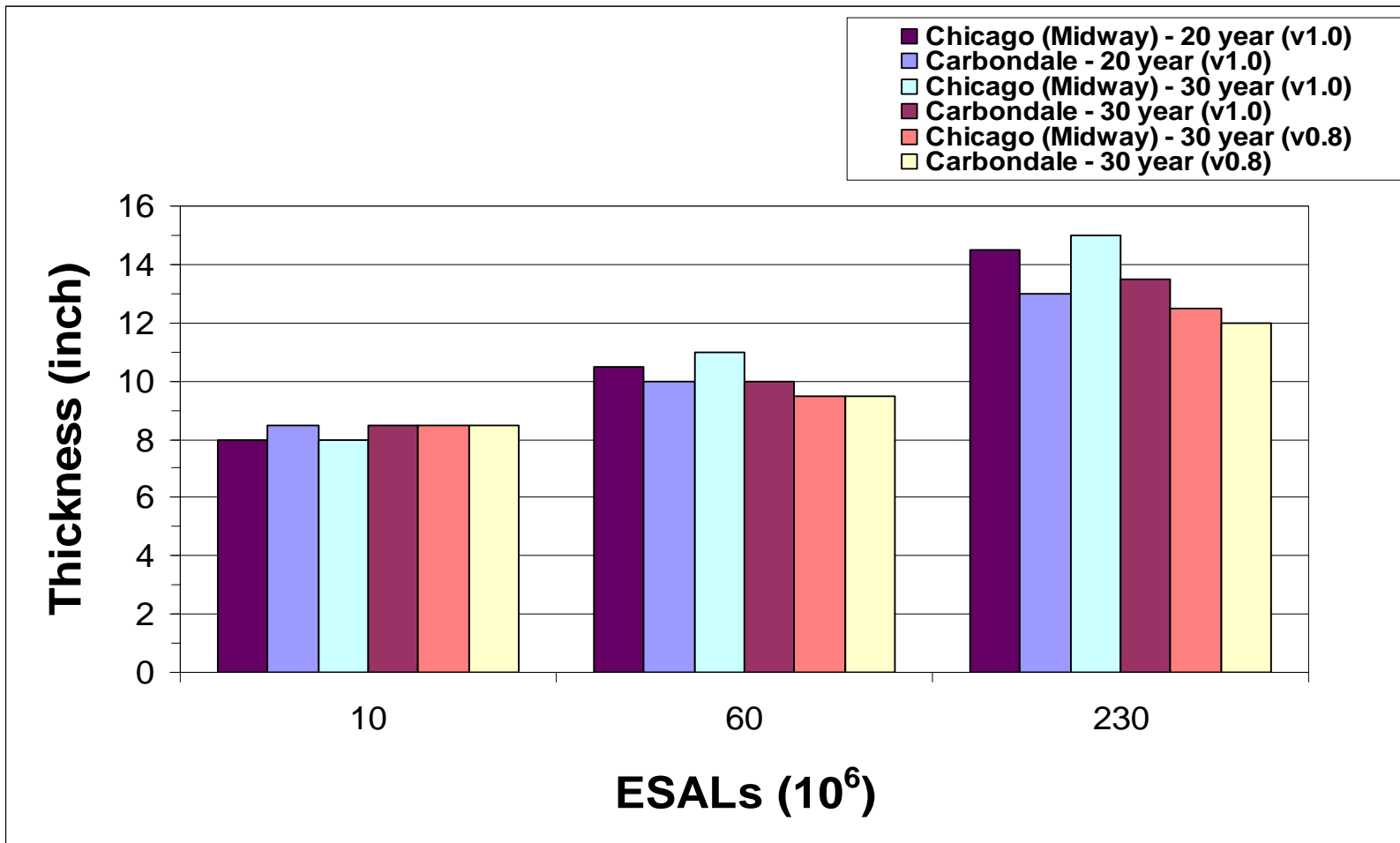


# CRCP with AC shoulder (M-EPDG)



**DUPAGE vs. Midway**

# CRCP with tied shoulder (separate)



# CRCP Summary

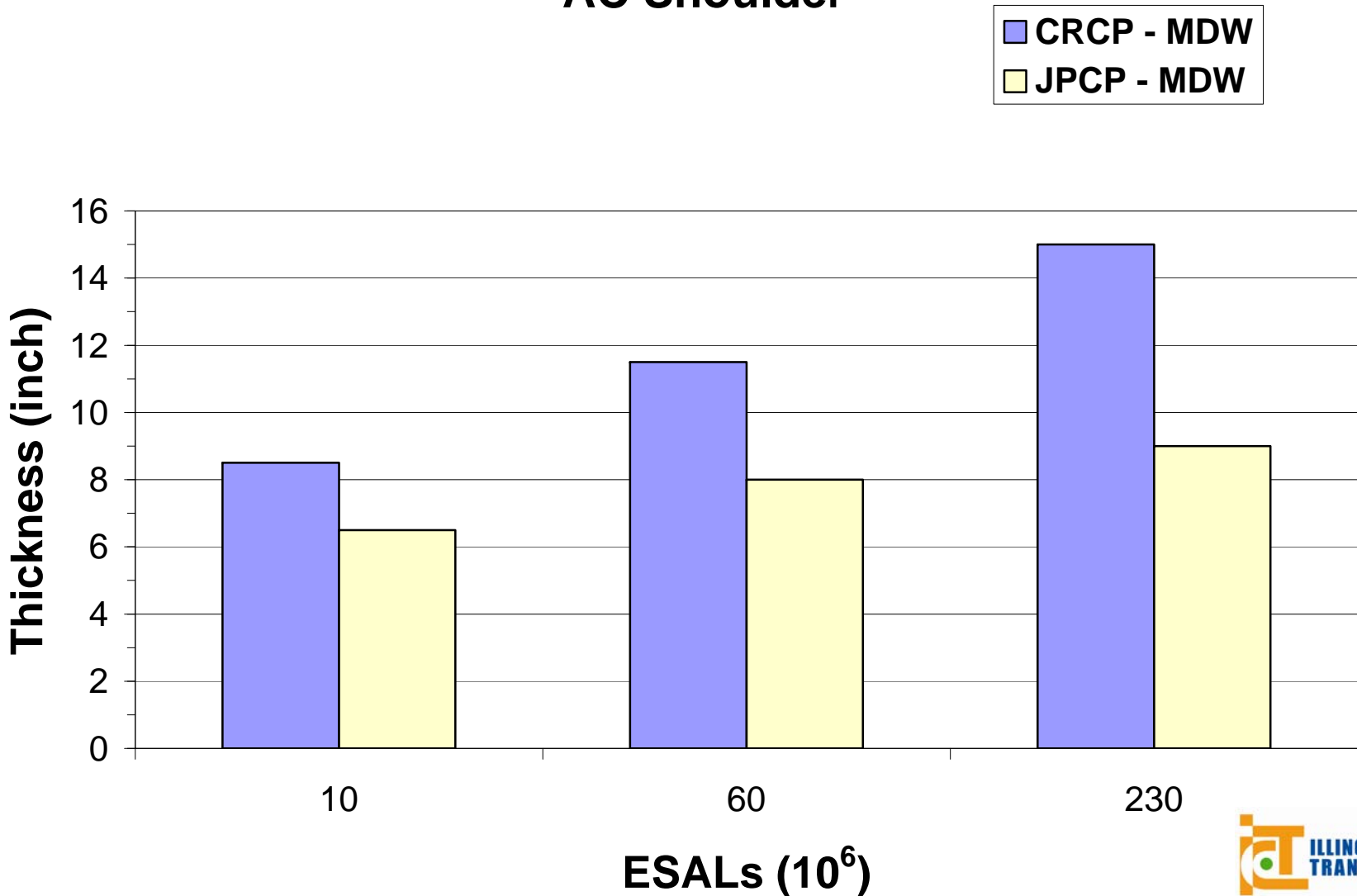
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- CRCP MEPDG w/ AC shoulder most similar to IDOT method
  
- Climate thickness effects
  - Midway > Dupage = Carbondale
  
- 30 year design gives greater thickness than 20 year design @ constant ESAL
  - Different steel content

v\_1.0

# JPCP vs. CRCP (MDW)

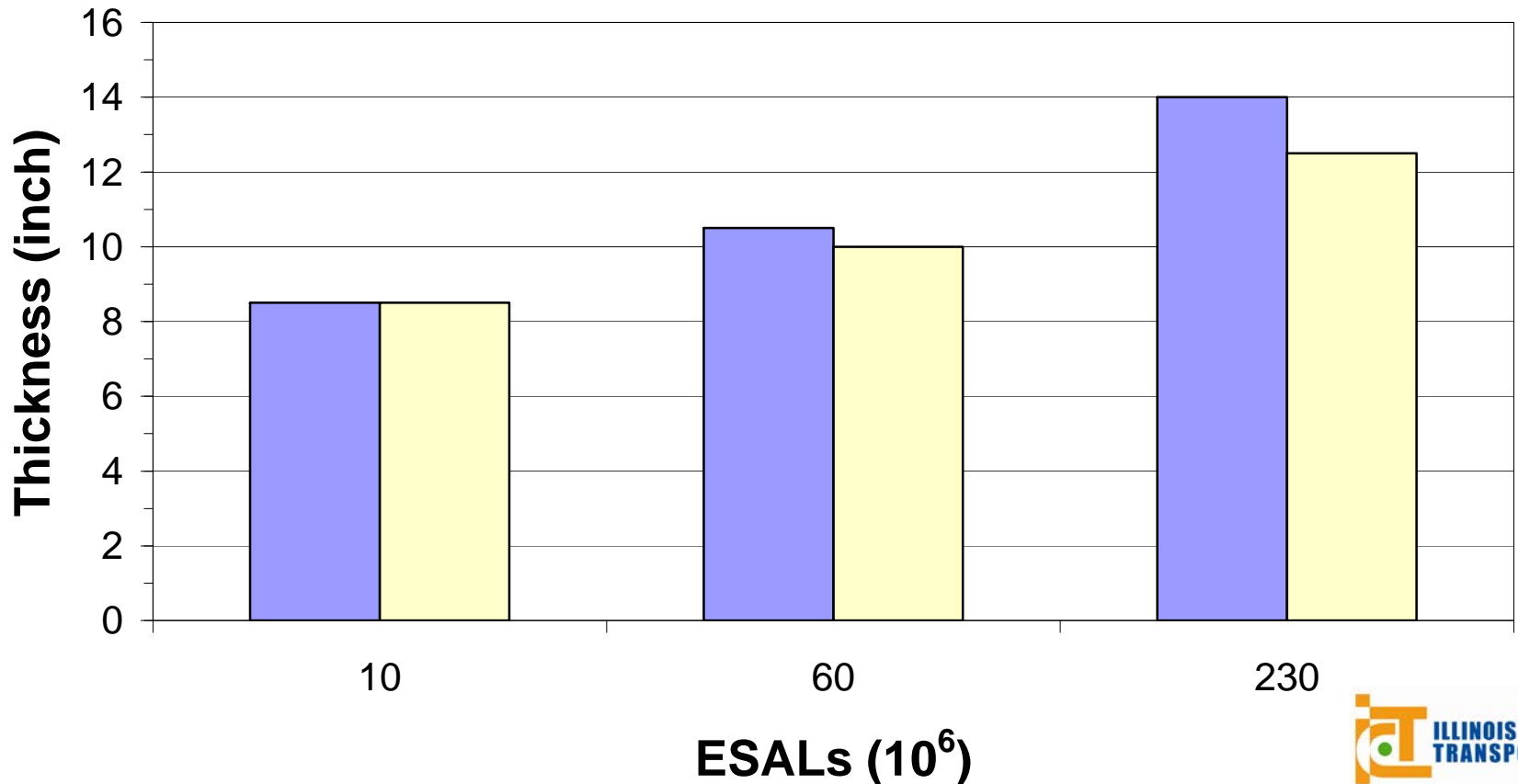
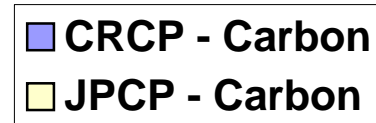
## AC Shoulder



v\_1.0

# JPCP vs. CRCP (Carbondale)

## AC Shoulder



# Summary

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- There is a temperature effect but difficult to make it into a *simple* statewide design method.
  
- For JPCP
  - use joint spacing specifications to account for climate changes
  
- For CRCP: initial construction temperatures very important!



# Potential - JPCP Calibration Data

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- Appendix FF – MEPDG
  - JPCP and CRCP
  - Traffic, % cracking, load spectra
  - SHRP Sections, RPRR, COPES
  - 516 JPCP observations
  
- IDOT video surveys

# MEPDG CRCP Calibration

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- 22 States w/ 4 climatic regions
  
- 58 CRCP sections
  - 10 sections from Illinois
  - Vandalia (US40), I-80, I-94 Edens – Heavy traffic

# Acknowledgements

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